Exhibit A

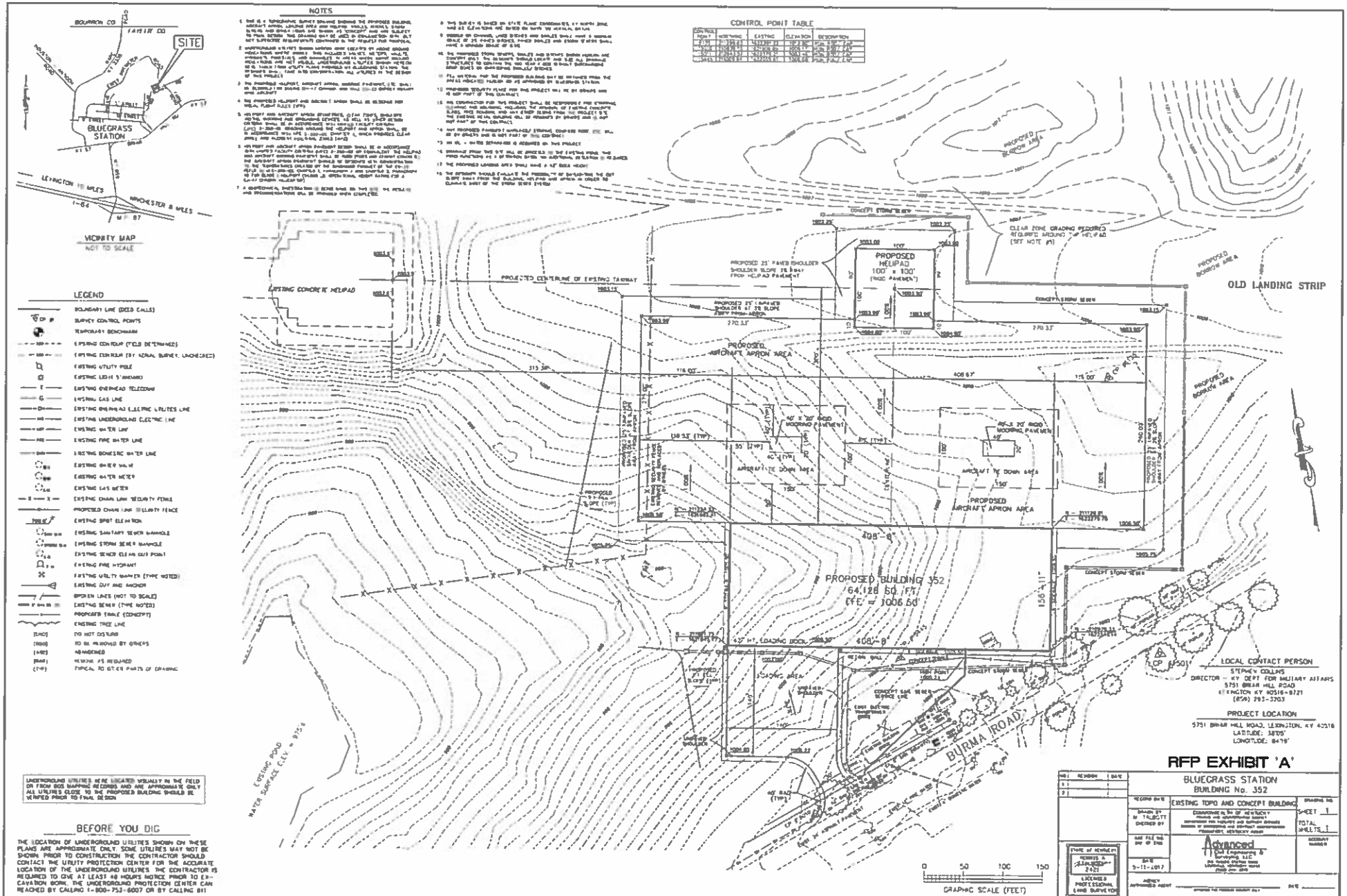

BOURBON CO
FAYETTE CO
SITE
BLUEGRASS STATION
LEXINGTON 15 MILES
WINCHESTER 8 MILES
I-64
VICINITY MAP
NOT TO SCALE
NOTES
CONTROL POINT TABLE
LEGEND
EXISTING CONCRETE HELIPAD
PROJECTED CENTERLINE OF EXISTING TAXIWAY
PROPOSED HELIPAD 100' x 100' (TAXI PAVEMENT)
PROPOSED AIRCRAFT APRON AREA
PROPOSED BUILDING 352
64,128 SQ. FT.
FFE = 1006.50'
OLD LANDING STRIP
BURMA ROAD
CONCEPT STORM SEWER
CLEAR ZONE GRADING REQUIRED AROUND 3' HELIPAD (SEE NOTE #5)
PROPOSED BORROW AREA
LOCAL CONTACT PERSON
STEPHEN COLLINS
DIRECTOR - KY DEPT FOR MILITARY AFFAIRS
5751 BRIAR HILL ROAD
LEXINGTON KY 40516-9721
(859) 293-3703
PROJECT LOCATION
5751 BRIAR HILL ROAD, LEXINGTON, KY 40516
LATITUDE: 38°05'
LONGITUDE: 84°19'
BEFORE YOU DIG
THE LOCATION OF UNDERGROUND UTILITIES SHOWN ON THESE PLANS ARE APPROXIMATE ONLY. SOME UTILITIES MAY NOT BE SHOWN. PRIOR TO CONSTRUCTION THE CONTRACTOR SHOULD CONTACT THE UTILITY PROTECTION CENTER FOR THE ACCURATE LOCATION OF THE UNDERGROUND UTILITIES. THE CONTRACTOR IS REQUIRED TO GIVE AT LEAST 48 HOURS NOTICE PRIOR TO EXCAVATION WORK. THE UNDERGROUND PROTECTION CENTER CAN REACHED BY CALLING 1-800-752-6007 OR BY CALLING 811
GRAPHIC SCALE (FEET)
SCALE 1" = 50'
RFP EXHIBIT 'A'
BLUEGRASS STATION
BUILDING No. 352
EXISTING TOPO AND CONCEPT BUILDING
Advanced
5-11-2017

Exhibit B

DEPARTMENT OF MILITARY AFFAIRS
BLUEGRASS STATION DIVISION

January 13, 2014

CONTRACTOR PROCEDURES GUIDE

GENERAL

Provisions contained in this guide are not intended to be all inclusive; however, adherence to the guidance provided is essential to safe and efficient operations during construction. Safety is of paramount importance, and all personnel are expected to abide by the rules set forth – further, individuals shall immediately report any unsafe act or condition(s) to the appropriate responsible individual and/or entity.

SAFETY AND ENVIRONMENTAL

- **The General Contractor** ***(GC)*** *(also includes sub-contractors, vendor, material deliveries, etc.)* shall check-in with BGS Maintenance Branch POC and/or the specific building occupant POC at the beginning and end of each shift. Bluegrass Station (BGS) will advise GC of any work schedule changes or restrictions taking place at any given time that may affect the work.
- Access to the interior of Buildings is restricted and shall be coordinated, cleared and authorized with BGS Officials and/or Building Occupant POC.
- GC and subcontractors' point of contact information (cell & office phones, etc.) shall be provided to BGS Officials for direct communications when required.
- Foreign Object Debris (FOD) Control – GC and subcontractors shall maintain a clean project site and remove all debris and secure all materials each work day.
- Location of portable toilets shall be coordinated with BGS Officials.
- Utilities Provided by BGS - GC, contractor personnel, prime and sub-contractors shall monitor usage of water and electricity and shall take all actions and precautions to prevent misuse and wasting of these resources.
- Entry points to and from the project site shall be designated by BGS Officials and/or Building Occupant POC.
- Authorized smoking areas will be designated by BGS Officials and/or Building Occupant POC.
  Smoking is NOT allowed anywhere inside any building. The GC will provide ash cans and be responsible for keeping the area clean.
- Hazardous Materials: Prior to the use of chemicals/substances, Material Safety Data Sheets (MSDS) shall be provided to BGS Officials for review and approval prior to use.
- Construction waste containers shall remain closed; no open top roll-away dumpsters are allowed unless they can be sufficiently maintained to prevent materials from being blown from the containers. All materials contained within containers is the property of Bluegrass Station and shall remain in the containers; "dumpster diving" is NOT allowed.
- Speed limit on Bluegrass Station property is 15 MPH.
- Contractor shall keep dust and dirt infiltration to a minimum and provide protection measures as necessary.
- The use of cranes shall be closely coordinated with BGS Officials.

- Construction Entrance – All equipment and material deliveries are required to enter at BGS Gate # 5, 3170 Houston-Antioch Road, Lexington, KY *(Alternate arrangements may be made at other BGS access points when needed.)*
- Excavation on BGS property – Underground Facility Damage Prevention: all requirements of the **Kentucky Call Before You Dig Law, KRS 367.4901 – 367.4917** shall be strictly adhered to. Each contractor or sub-contractor shall contact 811 in advance of any and all excavation activities (piggy-backing of requests by multiple contractors to locate are not allowed).

SECURITY

- General Contractor's personnel, prime and sub-contractors and suppliers/vendors on-site, shall be U.S. citizens.
- Entry to BGS property shall be controlled by its security personnel (Logan Security). Temporary vehicle passes will be issued by BGS security personnel to each person working at the project site. A list of all contractor personnel is required.
- All material and equipment deliveries are required to enter at Gate # 5 at 3170 Houston-Antioch Road, Lexington, KY *(Alternate arrangements may be made at other BGS access points when needed.)*
- Vehicles accessing Bluegrass Station property and the project site are subject to search by BGS Officials and/or Logan Security Guards upon requesting entry at Gates 1 & 5.
- The only vehicles permitted on the project site will be marked/labeled company vehicles. A list of all company vehicles shall be provided to BGS prior to being allowed on the project site. The contractor is encouraged to use company vehicles and carpool to limit the number of privately owned vehicles (POVs) on BGS property as parking is extremely limited. Contractors may be required to park in lot A, near the BGS Gate 1 entrance, and carpool from there to the project site.
- Off Limits Areas – All buildings/facilities within the fenced SOFSA/Lockheed Martin compound and other areas around or within any given building as defined unless prior access approval is received and coordinated.
- Restricted Areas - GC, contractor personnel, prime and sub-contractors shall refrain from "driving around" BGS property. Traveling is limited to access for daily parking and/or project site access.
- Photography is strictly prohibited; this includes the use of cell phones and all photographic devices. In the event the contractor must take project related photographs, they shall contact BGS for authorization and coordination prior to taking the photographs.
- Normal working hours are 7:00 a.m. – 4:30 p.m. however; other hours are negotiable through coordination with BGS Officials. Prior notice and approval is required for working after normal working hours or on weekends.

BGS Points of Contact:

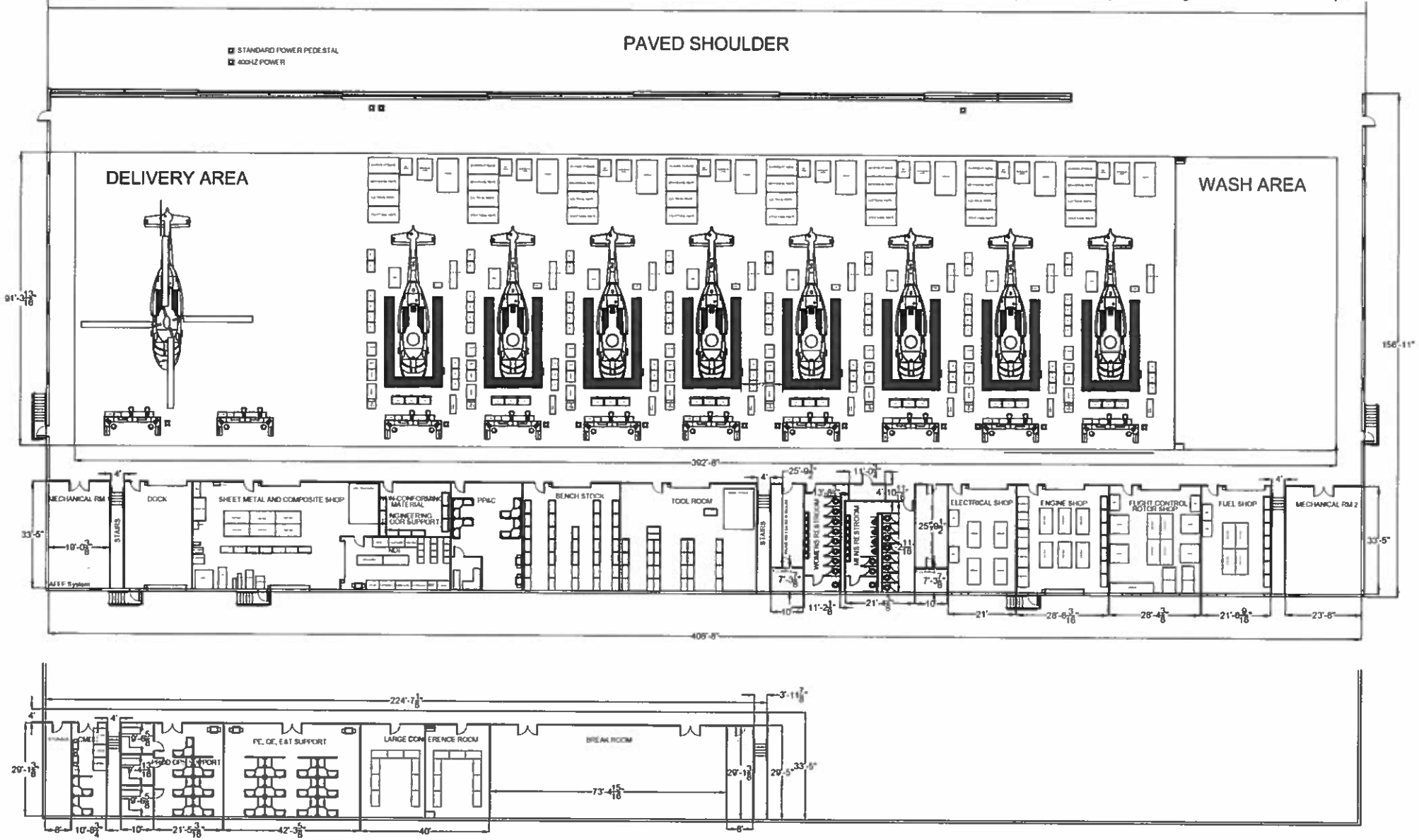
Paul T. Cable – 859.293.3203
David Jones – 859.293.3698
Andy Daigle – 859.293.4411

End

Exhibit C

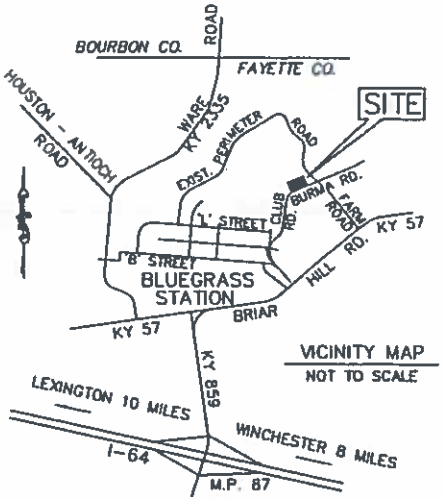
Exhibit C_v.4: Concept Drawing

Aircraft on drawing are for conceptual purposes only; model is subject to change based on mission requirements



BUILDING 352

Exhibit D



VICINITY MAP
NOT TO SCALE

LEASE BOUNDARY LINE TABLE

NUM	BEARING	DISTANCE
L-1	N76°29'50"W	116.00'
L-2	N13°30'10"E	260.00'
L-3	S76°29'50"E	270.33'
L-4	N13°30'10"E	90.00'
L-5	S76°29'50"E	100.00'
L-6	S13°30'10"W	90.00'
L-7	S76°29'50"E	270.33'
L-8	S13°30'10"W	260.00'
L-9	N76°29'50"W	116.00'
L-10	S13°30'10"W	156.92'
L-11	N76°29'50"W	268.67'
L-12	S13°30'10"W	137.60'
L-13	S71°56'11"W	110.04'
L-14	N18°06'51"W	17.82'
L-15	N76°29'50"W	76.94'
L-16	N13°30'10"E	296.92'

LEASE BOUNDARY CURVE TABLE

NUM	ARC	RADIUS	BEARING	DISTANCE
C-1	62.87'	40.00'	N26°54'40"E	56.59'
C-2	40.76'	40.00'	N47°18'20"W	39.02'
C-3	84.91'	40.00'	S47°18'20"E	69.84'

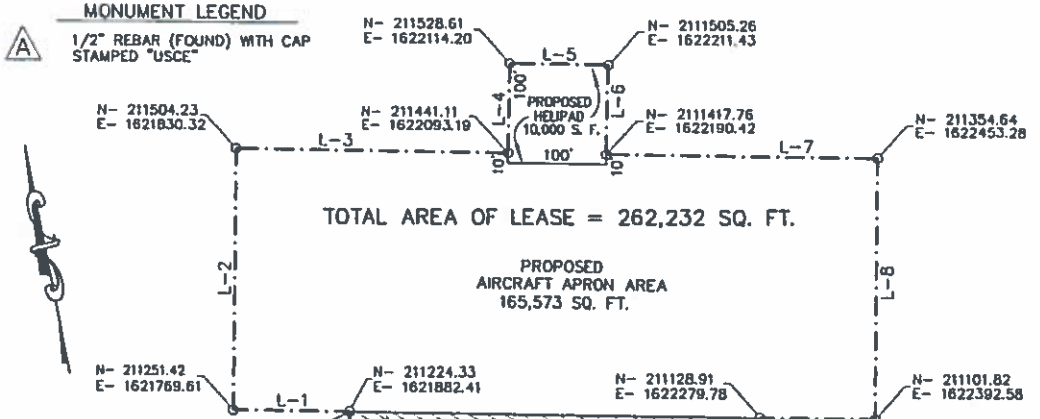
LEGEND

- COMM. OF KY BOUNDARY LINE
- LEASE BOUNDARY LINE (THIS SURVEY)
- "TIE" LINE
- BROKEN LINE

N= 208316.70' E= 1620404.17' STATE PLANE COORDINATES AT LEASE BOUNDARY CORNERS (KY NORTH ZONE) BASED ON NGS MONUMENT A-150 (PID HZ0130)

MONUMENT LEGEND

A 1/2" REBAR (FOUND) WITH CAP STAMPED "USCE"



N75°59'04"W 324.63'

'TIE' LINE S75°44'33"W 1152.61'

COMMONWEALTH OF KENTUCKY DEED BOOK 2835 PAGE 242

'TIE' LINE N64°47'20"E 810.31'

BOUNDARY LINE N58°22'11"W 407.91'

COMMONWEALTH OF KENTUCKY DEED BOOK 2713 PAGE 639

'TIE' LINE N62°45'58"E 849.54'

'TIE' LINE N17°00'08"E 187.95'

N16°20'31"W 29.98'

N16°30'28"E 381.49'

SURVEY CLASSIFICATION

THIS SURVEY AND PLAT MEETS OR EXCEEDS THE TECHNICAL STANDARDS FOR A CLASS 'B' SURVEY FOR RURAL LAND IN ACCORDANCE WITH 201 KAR 18.150.

REFERENCE BEARING

THE BEARINGS SHOWN HEREON ARE BASED ON GRID NORTH KY STATE PLANE COORDINATES, NORTH ZONE. NAD-83, GEOID 99, NAVD 1988.

STATEMENT ON PRECISION & MEASUREMENTS

THIS SURVEY WAS PERFORMED BY THE RANDOM TRAVERSE METHOD. THE UNADJUSTED LINEAR ERROR OF CLOSURE RATIO OF THE TRAVERSE EXCEEDS 1 PART IN 34690

LAND SURVEYORS CERTIFICATE

I HEREBY CERTIFY THAT THIS BOUNDARY SURVEY AND PLAT WERE MADE UNDER MY SUPERVISION AND THAT THE ANGULAR AND LINEAR MEASUREMENTS OF THE BOUNDARY LINES AS WITNESSED BY MONUMENTS SHOWN HEREON ARE TRUE AND CORRECT TO THE BEST OF MY KNOWLEDGE AND BELIEF. THIS SURVEY AND PLAT MEET OR EXCEEDS THE MINIMUM STANDARDS OF GOVERNING AUTHORITIES. THIS SURVEY AND PLAT IS ONLY FOR THE LEASE AREA AND THE PORTION OF BOUNDARY REFERENCED HEREON.

STATE of KENTUCKY
MORRIS A. TALBOTT
2421
LICENSED PROFESSIONAL LAND SURVEYOR

5-11-2017

EXCEPTIONS TO SURVEY

THIS LEASE AREA SURVEY IS SUBJECT TO ALL RIGHTS OF WAY, EASEMENTS, CONVEYANCES AND RESTRICTIONS THAT A TITLE EXAMINATION WOULD REVEAL. NO REPORT WAS PROVIDED TO THE SURVEYOR FOR PROJECT.

0 150 300
GRAPHIC SCALE (FEET)

EXHIBIT 'D'
PROPERTY PLAT

PLAT OF LEASE AREA SURVEY
BUILDING 352

COMMONWEALTH OF KENTUCKY
BLUEGRASS STATION DIVISION (OWNER & CLIENT)
5751 BRIAR HILL ROAD
LEXINGTON, KY 40516

TITLE REFERENCE D.B. 2835 PAGE 242

COUNTY OF FAYETTE, KENTUCKY

SITE ADDRESS: 5721 BRIAR HILL RD.

Advanced Engineering and Surveying, LLC
319 TUCKER STATION ROAD
LOUISVILLE, KENTUCKY 40243
(502) 244-3876

THIS PLAT REPRESENTS A PARTIAL BOUNDARY SURVEY AND COMPLIES WITH 201 KAR 18:150

PLAT SCALE: 1 INCH EQUALS 150 FEET.

BOUNDARY SURVEY DATE 12-14-2013

DRAWN 5-10-2017 FILE: C:\JOBS2017\BGS BLD 198 LEASE PLAT.DWG

Exhibit E

Report of Preliminary Geotechnical Services
Bluegrass Station Building 198
Lexington, Kentucky
S&ME Project No. 1183-17-020

S&ME

Prepared for:
Commonwealth of Kentucky Finance and Administration Cabinet
Department for Facilities and Support Services - Division of Engineering and Contract Administration
403 Wapping Street, 1st Floor
Frankfort, Kentucky 40601

Prepared by:
S&ME, Inc.
2020 Liberty Road, Suite 105
Lexington, Kentucky 40505

June 13, 2017

S&ME

June 13, 2017

Commonwealth of Kentucky Finance and Administration Cabinet
Department for Facilities and Support Services
Division of Engineering and Contract Administration
403 Wapping Street, 1st Floor
Frankfort, Kentucky 40601

Attention: Mr. Frank Phillips, P.E., CEM

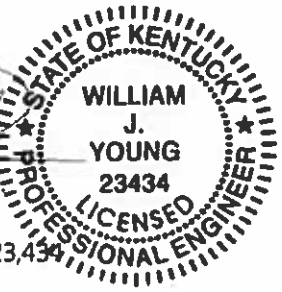
Reference: **Report of Preliminary Geotechnical Services**
Bluegrass Station Building 198
Lexington, Kentucky
S&ME Project No. 1183-17-020

Dear Mr. Phillips:

S&ME, Inc. has completed the preliminary geotechnical exploration for the proposed Building 198 to be constructed at Bluegrass Station in Avon, Kentucky. The purpose of this exploration was to obtain general subsurface data to assist in project development and planning. A design phase (final) geotechnical exploration is required once the project planning and development is more complete. We conducted this project in general accordance with S&ME Proposal No. 11-1700072 dated March 9, 2017 which was authorized by you. This report explains our understanding of the project, documents our findings, and presents our conclusions and geotechnical engineering considerations.

S&ME appreciates the opportunity to be of continued service to the Commonwealth of Kentucky and Bluegrass Station. We look forward to serving as your geotechnical engineering consultant through project completion. If you have any questions, please call.

Sincerely,
S&ME, Inc.



William J. Young, P.E.
Project Engineer
Licensed Kentucky, 23,434

Craig S. Lee, P.E.
Senior Engineer/Vice President

S&ME

Table of Contents

Appendices

1.0 INTRODUCTION

S&ME, Inc. has completed the preliminary geotechnical exploration for the proposed Building 198 to be constructed at Bluegrass Station in Avon, Kentucky. The purpose of this exploration was to obtain general subsurface data to assist in project development and planning. A design phase (final) geotechnical exploration is required once the project planning and development is more complete. We conducted this project in general accordance with S&ME Proposal No. 11-1700072 dated March 9, 2017 which was authorized by you. This report explains our understanding of the project, documents our findings, and presents our conclusions and geotechnical engineering considerations.

2.0 PROJECT AND SITE DESCRIPTION

The project site is located at the Bluegrass Station in Avon, Kentucky. Please reference Figure 1 in Appendix A for details. The project area is predominantly a grass-covered field with some construction debris to the south of the proposed building footprint. Provided topographic information indicates that elevations across the planned building area range from about 994 feet on the west side to about 1009 feet on the east side.

The project will consist of the "design-build" construction of a warehouse style building (Building 198) and associated pavement areas. The building will be a pre-engineered metal building approximately 157 feet wide and 409 feet long (approximately 64,128 square feet) with a grade supported concrete floor slab. Proposed finished floor elevation for the building is 1006.5 feet. Structural loading and settlement criteria were not available at the time of this report.

3.0 SITE GEOLOGY

The Geologic Map of the Clintonville Quadrangle, Kentucky, (GQ-717, 1968) published by the U.S. Geological Survey indicates the site is underlain by the Millersburg Member of the Lexington Limestone of the Ordovician Geologic Age. The Millersburg Member consists of limestone and shale, interbedded. Limestone (50 percent) is medium to dark gray, argillaceous, micro-grained, and occurs in thin beds with flat bedding planes. The shale is medium to dark gray, thin bedded to laminated, and weathers to yellowish clay soil. The results of our exploration confirm the geologic mapping for this area.

All areas underlain by potentially soluble rock (i.e. – limestone or dolomite) are at some risk due to sinkhole activity. No obvious sinkholes or closed depressions were observed on the site; however, portions of the site have been regraded. Thus, any surface evidence of solution activity may have been filled or destroyed during site grading in these areas. The Lexington Limestone is prone to differential weathering and solutioning. The result is an erratic top of rock profile with open fractures, cavities, and channels in the bedrock. Most of the Members of the Lexington Limestone are prone to form sinkholes. However, the Millersburg Member is not known to be sinkhole prone.

No obvious sinkholes were noted on the USGS topographic map or the USGS geologic map within one mile of the site. While the Kentucky Geological Survey (KGS) defines this area of Fayette County as having a high potential for Karst activity, our experience with the Millersburg Member indicates that sinkhole activity is not a major concern, but some localized solutioning and crevices can be expected. As such, we

believe there is a moderate risk associated with Karst activity at this site. The southwest-northeast trending Bryan Station Fault Zone lies about 3 miles to the northwest of this site.

Regional dip in the area is to the northwest at less than one percent. The significance of the regional dip is that the dip generally corresponds to the direction of subsurface water flow.

4.0 EXPLORATION METHODS

The procedures used by S&ME for field and laboratory sampling and testing are in general accordance with ASTM procedures and established engineering practice. Appendix II contains brief descriptions of the drilling and sampling procedures used in this exploration.

4.1 Field Exploration

A total of six (6) soil test borings were advanced for this exploration. The building corners were staked by Advance Engineering and Surveying prior to our exploration. Borings B-3, B-4, and B-6 were located at the building corners. Borings B-2 and B-5 were located in the field by estimating angles and taping distances from the building corner stakes. Boring B-1 was located in the tree/brush line and was offset from the building corner location. As such, the boring locations shown on Figure 2 should be considered approximate. The top of boring elevations were interpolated to the nearest one foot from the provided site topographic plan.

The borings were advanced using a track-mounted Diedrich D-50 drill rig using 6 7/8 inch O.D. hollow-stem augers. The drillers obtained soil samples in the soil test borings using a split-barrel sampler driven by an automatic hammer system in general accordance with ASTM D1586. Rock coring was beyond the scope of this preliminary exploration. The stratification lines shown on the Test Boring Records represent the approximate boundaries between soil and/or rock surfaces. The transitions may be more gradual than shown.

4.2 Laboratory Testing

The S&ME staff professional sealed and returned the soil samples to our laboratory where they assigned the applicable laboratory tests. These tests are used to determine the engineering properties of the soil. The soil samples were visually classified by the engineer in general accordance with the Unified Soil Classification System (ASTM D2487) during the field exploration.

We performed natural moisture content and Atterberg Limits tests on three representative soil samples from the borings. Atterberg Limits testing indicated Liquid Limits (LL) of 49 to 76 percent with Plasticity Indices (PI) of 27 to 47 percent classifying the tested samples as low plasticity (lean) clay (CL). The natural moisture content tests ranged from approximately 17 to 32 percent. The obtained laboratory data and descriptions of these tests are included in Appendix III.

Unconfined compressive strength tests were performed on two samples obtained from relatively undisturbed (Shelby) tube samples recovered during the field exploration. The results of the unconfined compressive strength testing are summarized in Table No. 1:

Table No. 1 – Unconfined Compressive Strength Testing

Boring	Soil Description	USCS Classification	Unconfined Compressive Strenth
B-1	Fat Brown Clay	CH	4,586
B-5	Fat Brown Clay	CH	4,567

5.0 SUBSURFACE CONDITIONS

5.1 General Soil Profile

The borings were drilled through existing grass areas and encountered approximately six to ten inches of topsoil. Beneath the existing topsoil, predominantly high plasticity residual clay was encountered in each of the borings. The sample that classified as lean clay (CL) in boring B-3 is within one percent of the Liquid Limit criterion for being classified as high plasticity (fat) clay and will, in our judgment, behave as such. The borings encountered auger refusal between depths of 5.0 and 10.5 feet. The consistency of these soils varied from firm to very stiff. Natural moisture contents of the high plasticity (fat) clay (CH) averaged about 26 percent. Atterberg limits tests of the lean clay at boring B-3 indicated a liquid limit range of 49 percent with a plasticity index of 27 percent. Atterberg limits tests of the fat clay indicated a liquid limit range of 72 to 76 percent with a plasticity index range of 46 to 47 percent. Our borings were terminated upon auger refusal, which we interpreted as bedrock.

Please refer to the Test Boring Records in Appendix II for details.

5.2 Groundwater

The borings were dry upon completion of drilling. The borings were backfilled with auger cuttings after the completion of drilling. As such, 24-hour water levels were not measured. Management of both surface and subsurface water will be a key issue to development of this site. Please refer to the following section of this report for additional details and recommendations regarding water management.

6.0 GEOTECHNICAL CONSIDERATIONS

We identified the following key issues that may impact the development of the site:

6.1 Grading and Site Selection

Site grading plans were not available at the time of this report. As such, we do not know the extent or magnitude of grading planned. We were provided a concept drawing with a proposed Finished Floor Elevation (FFE) of 1006.50. The depth to rock in the explored areas ranged from about 5.0 to 10.5 feet below existing grades.

The site grading plan should take into account the following:

- Provided drawings indicate a FFE of 1006.50, and existing site grades ranging from 984 to 1009 feet. As such, we expect cut depths up to five feet and fill depths up to two and one half feet to achieve the desired grades.

- The encountered bedrock elevations in our borings trend upward from west to east. Based on the concept drawing FFE of 1006.50 bedrock may be encountered in utility excavation to the east of the building.

6.2 High Plasticity Soils

Visual identification of samples and Atterberg limits testing performed during this preliminary exploration indicate that the soil is comprised predominantly of high plasticity fat clay (CH). Soils with plasticity indices greater than 35 percent have a high risk of experiencing shrink or swell with changes in moisture content. Lightly loaded structural elements such as slabs-on-grade, sidewalks, pavement areas and non-load bearing walls are most susceptible to damage from shrinking and swelling soils. The final geotechnical exploration should include additional plasticity testing and swell testing to further define the engineering properties of the soil, and to assess the magnitude that the fat clay will impact development prior to implementing costly procedures to mitigate the plasticity issue.

6.3 Construction Accessibility / Site Degradation

Based on our on-site observations and our past experience with similar soil conditions, construction accessibility will be problematic if attempted during cold/wet seasons of the year. Construction accessibility should be better during the hot/drier months of the year. During the wet periods, a construction road or pad consisting of a geo-textile fabric overlain by gravel may be required. Soft and/or wet areas may require selective undercutting, repair after construction is completed, or other treatment as recommended by the geotechnical engineer. We recommend that this site be graded and developed during warm, dry months of the year.

6.4 Water Management

Management of both surface and subsurface water will be a key issue to development of this site. We anticipate that the low lying areas will be the most problematic with respect to water. Subsurface water will always tend to migrate toward the low lying areas, even after the site grading has been performed. Phase the earthwork such that the low lying areas are stabilized and are able to convey water away from the site while maintaining the integrity of the site.

6.5 Use of On-site Soils

We define acceptable structural soil fill as inorganic natural soil with maximum particles sizes of 3 inches, a plasticity index of 30 or less, and maximum dry density of at least 100 pounds per cubic foot (pcf) when tested by the standard Proctor method (ASTM D698).

Our work scope did not include standard Proctor testing. However, correlations between the Optimum Moisture Content (OMC) obtained from standard Proctor testing and the soils Plastic Limit (PL) indicate

soils in Central Kentucky are typically within two to three percent of each other. Based on the Atterberg Limits testing and Natural Moisture Content of the tested samples from our May 19, 2017 exploration, we do not anticipate that moisture conditioning (wetting or drying) will be required. If grading occurs during either wetter or dryer periods of the year, moisture conditioning may be required.

The results of our preliminary exploration indicate that some of the encountered fat clay (CH) soils do not meet the plasticity criteria for structural soil fill. Additional testing of both the plasticity and swell testing should be performed during the design phase exploration. If the testing indicates the fat clay is problematic, we will recommend that soils with a plasticity index greater than 30 percent be kept at least three feet below subgrade under buildings and sidewalks adjacent to the buildings. The fat clays may be used in subgrade elevations in lawn and parking lot (not concrete) areas, and deeper than three feet below subgrade in the building pad.

During the geotechnical design phase, standard Proctor testing and Atterberg limits testing of potential fill soils (on-site and/or off-site) should be performed by S&ME before they are used as fill material. If off-site fill is imported, we recommend that the proposed borrow soil be tested prior to transporting it to the site. Please realize that the laboratory conformance testing usually takes three to four business days to complete.

6.6 Foundations and Floor Slabs

We anticipate conventional spread footings bearing on the firm or better soils or newly placed and compacted structural soil fill may be used for the proposed structures.

6.7 Future Studies

The above items warrant further attention and should be addressed on a more detailed design phase exploration program. Additionally, the design phase geotechnical exploration should address the following, at a minimum:

- Soil test borings within the proposed building footprint to develop foundation and floor slab (where slab-on-grade floors are planned) design recommendations.
- Standard Proctor tests of proposed fill soil.
- CBR tests for asphalt/concrete roadway construction.
- If some areas will receive more than six feet of new soil fill, the design phase geotechnical exploration should include detailed settlement studies.
- Additional plasticity testing and swell testing should be performed to define the potential impact of expansive clays prior to excluding their use within the top three feet.

We anticipate a site seismic classification of "C" based on our limited borings. It is possible that a site specific seismic evaluation would allow for a less conservative structural design and realized construction cost savings.

Generally, there should be a more concentrated spread of borings for buildings, and at least one boring per 10,000 to 15,000 square feet of parking areas. The number and depth of borings will vary for each proposed structure depending on its size, type, and proposed use.

7.0 FOLLOW UP SERVICES

This report is preliminary and is not intended for final design purposes. Additional geotechnical work will be required once specific building location, type, and grades have been established.

8.0 LIMITATIONS

This report has been prepared for the exclusive use of the Commonwealth of Kentucky for specific application to this project site. Our conclusions and recommendations have been prepared using generally accepted standards of geotechnical engineering practice in the Commonwealth of Kentucky. No other warranty is expressed or implied. This company is not responsible for the conclusions, opinions, or recommendations of others based on these data.

Our conclusions and recommendations are based on the design information furnished to us, the data obtained from the previously described preliminary geotechnical exploration, and our past experience. They do not reflect variations in the subsurface conditions that are likely to exist between our borings and in unexplored areas of the site. These variations result from the inherent variability of the general subsurface conditions in this geologic region.

We recommend that the Owner retain S&ME to continue our involvement in the project through the subsequent phases of design and construction. Our firm is not responsible for interpretation of the data contained in this report by others.

Important Information about This

Geotechnical-Engineering Report

Subsurface problems are a principal cause of construction delays, cost overruns, claims, and disputes.

While you cannot eliminate all such risks, you can manage them. The following information is provided to help.

Geotechnical Services Are Performed for Specific Purposes, Persons, and Projects

Geotechnical engineers structure their services to meet the specific needs of their clients. A geotechnical-engineering study conducted for a civil engineer may not fulfill the needs of a constructor — a construction contractor — or even another civil engineer. Because each geotechnical- engineering study is unique, each geotechnical-engineering report is unique, prepared *solely* for the client. No one except you should rely on this geotechnical-engineering report without first conferring with the geotechnical engineer who prepared it. *And no one — not even you* — should apply this report for any purpose or project except the one originally contemplated.

Read the Full Report

Serious problems have occurred because those relying on a geotechnical-engineering report did not read it all. Do not rely on an executive summary. Do not read selected elements only.

Geotechnical Engineers Base Each Report on a Unique Set of Project-Specific Factors

Geotechnical engineers consider many unique, project-specific factors when establishing the scope of a study. Typical factors include: the client's goals, objectives, and risk-management preferences; the general nature of the structure involved, its size, and configuration; the location of the structure on the site; and other planned or existing site improvements, such as access roads, parking lots, and underground utilities. Unless the geotechnical engineer who conducted the study specifically indicates otherwise, do not rely on a geotechnical-engineering report that was:

- not prepared for you;
- not prepared for your project;
- not prepared for the specific site explored; or
- completed before important project changes were made.

Typical changes that can erode the reliability of an existing geotechnical-engineering report include those that affect:

- the function of the proposed structure, as when it's changed from a parking garage to an office building, or from a light-industrial plant to a refrigerated warehouse;
- the elevation, configuration, location, orientation, or weight of the proposed structure;
- the composition of the design team; or
- project ownership.

As a general rule, *always* inform your geotechnical engineer of project changes—even minor ones—and request an assessment of their impact. *Geotechnical engineers cannot accept responsibility or liability for problems that occur because their reports do not consider developments of which they were not informed.*

Subsurface Conditions Can Change

A geotechnical-engineering report is based on conditions that existed at the time the geotechnical engineer performed the study. *Do not rely on a geotechnical-engineering report whose adequacy may have been affected by:* the passage of time; man-made events, such as construction on or adjacent to the site; or natural events, such as floods, droughts, earthquakes, or groundwater fluctuations. *Contact the geotechnical engineer before applying this report to determine if it is still reliable.* A minor amount of additional testing or analysis could prevent major problems.

Most Geotechnical Findings Are Professional Opinions

Site exploration identifies subsurface conditions only at those points where subsurface tests are conducted or samples are taken. Geotechnical engineers review field and laboratory data and then apply their professional judgment to render an opinion about subsurface conditions throughout the site. Actual subsurface conditions may differ — sometimes significantly — from those indicated in your report. Retaining the geotechnical engineer who developed your report to provide geotechnical-construction observation is the most effective method of managing the risks associated with unanticipated conditions.

A Report's Recommendations Are Not Final

Do not overrely on the confirmation-dependent recommendations included in your report. *Confirmation-dependent recommendations are not final,* because geotechnical engineers develop them principally from judgment and opinion. Geotechnical engineers can finalize their recommendations *only* by observing actual subsurface conditions revealed during construction. *The geotechnical engineer who developed your report cannot assume responsibility or liability for the report's confirmation-dependent recommendations if that engineer does not perform the geotechnical-construction observation required to confirm the recommendations' applicability.*

A Geotechnical-Engineering Report Is Subject to Misinterpretation

Other design-team members' misinterpretation of geotechnical-engineering reports has resulted in costly

problems. Confront that risk by having your geotechnical engineer confer with appropriate members of the design team after submitting the report. Also retain your geotechnical engineer to review pertinent elements of the design team's plans and specifications. Constructors can also misinterpret a geotechnical-engineering report. Confront that risk by having your geotechnical engineer participate in prebid and preconstruction conferences, and by providing geotechnical construction observation.

Do Not Redraw the Engineer's Logs

Geotechnical engineers prepare final boring and testing logs based upon their interpretation of field logs and laboratory data. To prevent errors or omissions, the logs included in a geotechnical-engineering report should *never* be redrawn for inclusion in architectural or other design drawings. Only photographic or electronic reproduction is acceptable, *but recognize that separating logs from the report can elevate risk.*

Give Constructors a Complete Report and Guidance

Some owners and design professionals mistakenly believe they can make constructors liable for unanticipated subsurface conditions by limiting what they provide for bid preparation. To help prevent costly problems, give constructors the complete geotechnical-engineering report, *but* preface it with a clearly written letter of transmittal. In that letter, advise constructors that the report was not prepared for purposes of bid development and that the report's accuracy is limited; encourage them to confer with the geotechnical engineer who prepared the report (a modest fee may be required) and/or to conduct additional study to obtain the specific types of information they need or prefer. A prebid conference can also be valuable. *Be sure constructors have sufficient time* to perform additional study. Only then might you be in a position to give constructors the best information available to you, while requiring them to at least share some of the financial responsibilities stemming from unanticipated conditions.

Read Responsibility Provisions Closely

Some clients, design professionals, and constructors fail to recognize that geotechnical engineering is far less exact than other engineering disciplines. This lack of understanding has created unrealistic expectations that have led to disappointments, claims, and disputes. To help reduce the risk of such outcomes, geotechnical engineers commonly include a variety of explanatory provisions in their reports. Sometimes labeled "limitations," many of these provisions indicate where geotechnical engineers' responsibilities begin and end, to help others recognize their own responsibilities and risks. *Read these provisions closely.* Ask questions. Your geotechnical engineer should respond fully and frankly.

Environmental Concerns Are Not Covered

The equipment, techniques, and personnel used to perform an *environmental* study differ significantly from those used to perform a *geotechnical* study. For that reason, a geotechnical-engineering report does not usually relate any environmental findings, conclusions, or recommendations; e.g., about the likelihood of encountering underground storage tanks or regulated contaminants. *Unanticipated environmental problems have led to numerous project failures.* If you have not yet obtained your own environmental information, ask your geotechnical consultant for risk-management guidance. *Do not rely on an environmental report prepared for someone else.*

Obtain Professional Assistance To Deal with Mold

Diverse strategies can be applied during building design, construction, operation, and maintenance to prevent significant amounts of mold from growing on indoor surfaces. To be effective, all such strategies should be devised for the *express purpose* of mold prevention, integrated into a comprehensive plan, and executed with diligent oversight by a professional mold-prevention consultant. Because just a small amount of water or moisture can lead to the development of severe mold infestations, many mold- prevention strategies focus on keeping building surfaces dry. While groundwater, water infiltration, and similar issues may have been addressed as part of the geotechnical- engineering study whose findings are conveyed in this report, the geotechnical engineer in charge of this project is not a mold prevention consultant; *none of the services performed in connection with the geotechnical engineer's study were designed or conducted for the purpose of mold prevention. Proper implementation of the recommendations conveyed in this report will not of itself be sufficient to prevent mold from growing in or on the structure involved.*

Rely, on Your GBC-Member Geotechnical Engineer for Additional Assistance

Membership in the Geotechnical Business Council of the Geoprofessional Business Association exposes geotechnical engineers to a wide array of risk-confrontation techniques that can be of genuine benefit for everyone involved with a construction project. Confer with you GBC-Member geotechnical engineer for more information.

GBC GEOTECHNICAL BUSINESS COUNCIL
of the Geoprofessional Business Association

8811 Colesville Road/Suite G106, Silver Spring, MD 20910
Telephone: 301/565-2733 Facsimile: 301/589-2017
e-mail: info@geoprofessional.org www.geoprofessional.org

Appendix I– Site Location Plan / Boring Location Plan

SCALE:	1" = 2000'
DATE:	6/9/2016
DRAWN BY:	LHR
PROJECT NO:	1183-17-020

S&ME
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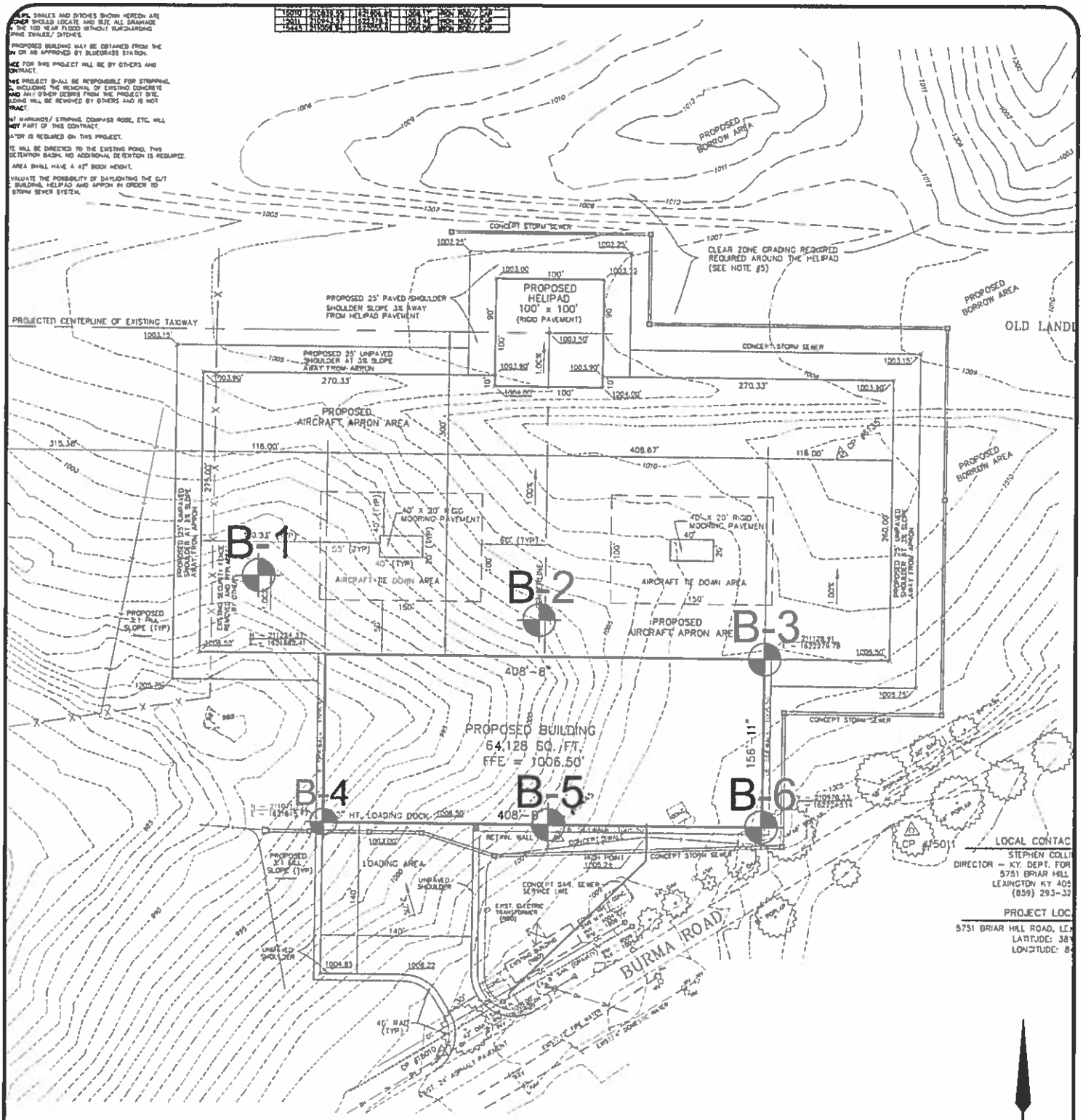
2020 LIBERTY ROAD, SUITE 105
LEXINGTON, KENTUCKY 40505
PHONE: 859-293-5518

VICINITY MAP
BLUEGRASS STATION
BUILDING 198
AVON, FAYETTE COUNTY, KENTUCKY

FIGURE NO.
1



LEGEND

Boring Location ⊕ B - X

Note: Survey by Advanced Civil Engineering & Surveying, LLC sheet 1 of 1 dated 5-11-2017.

SCALE:	1" = 100'
DATE:	6/9/2016
DRAWN BY:	LHR
PROJECT NO:	1183-17-020

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LEXINGTON, KENTUCKY 40505
PHONE: 859-293-5518

BUILDING SITE
BLUEGRASS STATION
BUILDING 198
AVON, FAYETTE COUNTY, KENTUCKY

FIGURE NO. 2

Appendix II – Test Boring Records

TEST BORING RECORD LEGEND

FINE AND COARSE GRAINED SOIL INFORMATION

COARSE GRAINED SOILS (SANDS & GRAVELS)

N	Relative Density
0-4	Very Loose
5-10	Loose
11-20	Firm
21-30	Very Firm
31-50	Dense
Over 50	Very Dense

FINE GRAINED SOILS (SILTS & CLAYS)

N	Consistency	Qu, KSF Estimated
0-1	Very Soft	0-0.5
2-4	Soft	0.5-1
5-8	Firm	1-2
9-15	Stiff	2-4
16-30	Very Stiff	4-8
Over 31	Hard	8+

PARTICLE SIZE

Boulders	Greater than 300 mm (12 in)
Cobbles	75 mm to 300 mm (3 to 12 in)
Gravel	4.74 mm to 75 mm (3/16 to 3 in)
Coarse Sand	2 mm to 4.75 mm
Medium Sand	0.425 mm to 2 mm
Fine Sand	0.075 mm to 0.425 mm
Silts & Clays	Less than 0.075 mm

The **STANDARD PENETRATION TEST** as defined by ASTM D 1586 is a method to obtain a disturbed soil sample for examination and testing and to obtain relative density and consistency information. A standard 1.4-inch I.D./2-inch O.D. split-barrel sampler is driven three 6-inch increments with a 140 lb. hammer falling 30 inches. The hammer can either be of a trip, free-fall design, or actuated by a rope and cathead. The blow counts required to drive the sampler the final two increments are added together and designate the N-value defined in the above tables.

ROCK PROPERTIES

ROCK QUALITY DESIGNATION (RQD)

Percent RQD	Quality
0-25	Very Poor
25-50	Poor
50-75	Fair
75-90	Good
90-100	Excellent

ROCK HARDNESS

Very Hard:	Rock can be broken by heavy hammer blows.
Hard:	Rock cannot be broken by thumb pressure, but can be broken by moderate hammer blows.
Moderately Hard:	Small pieces can be broken off along sharp edges by considerable hard thumb pressure; can be broken with light hammer blows.
Soft:	Rock is coherent but breaks very easily with thumb pressure at sharp edges and crumbles with firm hand pressure.
Very Soft:	Rock disintegrates or easily compresses when touched; can be hard to very hard soil.

$$\text{Recovery} = \frac{\text{Length of Rock Core Recovered}}{\text{Length of Core Run}} \times 100$$

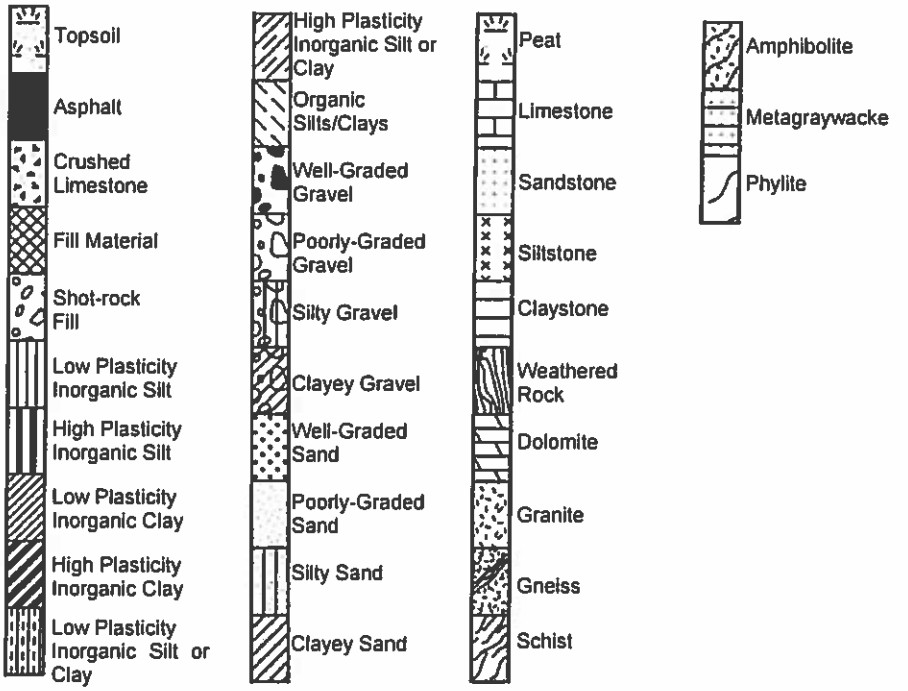
63 REC
NQ
43 RQD

$$\text{RQD} = \frac{\text{Sum of 4 in. and longer Rock Pieces Recovered}}{\text{Length of Core Run}} \times 100$$

Core Diameter	Inches
BQ	1-7/16
NQ	1-7/8
HQ	2-1/2

SYMBOLS

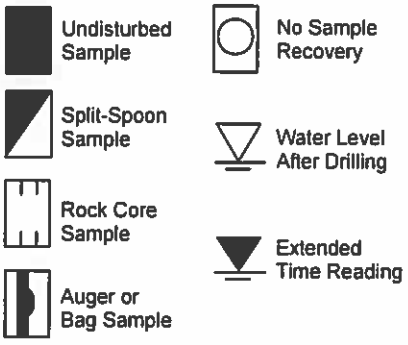
KEY TO MATERIAL TYPES



SOIL PROPERTY SYMBOLS

N:	Standard Penetration, BPF
M:	Moisture Content, %
LL:	Liquid Limit, %
PI:	Plasticity Index, %
Qp:	Pocket Penetrometer Value, TSF
Qu:	Unconfined Compressive Strength Estimated Qu, TSF
γ_D:	Dry Unit Weight, PCF
F:	Fines Content

SAMPLING SYMBOLS



S&ME

TEST BORING RECORD

BORING NO: **B-1**

PROJECT: Building 198 Bluegrass Station	JOB NO: 1183-17-020	REPORT NO:
PROJECT LOCATION: Lexington, KY		
ELEVATION: 996.0	BORING STARTED: 5/19/2017	BORING COMPLETED: 5/19/2017
DRILLING METHOD: 4" HSA	RIG TYPE: D-50	HAMMER: Automatic
GROUNDWATER (ft): Dry upon completion of soil augering	BORING DIAMETER (IN): 4	SHEET 1 OF 1

Remarks: Elevations interpolated from provided topographic mapping to nearest whole foot

Groundwater	ELEV. (FT.)	DEPTH (FT.)	MATERIAL DESCRIPTION	Lithology	Sample Type	Recovery (in)	RQD (%)	Qu	STANDARD PENETRATION RESISTANCE (N) 0 10 20 30 40 60	BLOWS /6"
	996	0	Topsoil 10 inches							
	995.1		Fat clay (CH), with black oxide nodules, FIRM, brown, moist			18				3 - 4 - 4
	994.5		Fat clay (CH), with black oxide nodules, STIFF to VERY STIFF, brown with gray mottling, moist			24		4,586		
		5				18				4 - 4 - 6
	990.5		Fat clay (CH), with black oxide nodules, with rock fragments, STIFF to VERY STIFF, brown with gray mottling, moist			18				4 - 9 - 10
	987.2		Auger refusal encountered at 8.8 feet			3				50/3
		10								
		15								
		20								

CRAIG2 1183-17-020 BLDG 198 BLUEGRASS STATION.GPJ QOR_CORP.GDT 6/13/17

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TEST BORING RECORD

BORING NO: **B-2**

PROJECT: Building 198 Bluegrass Station	JOB NO: 1183-17-020	REPORT NO:
PROJECT LOCATION: Lexington, KY		
ELEVATION: 1,003.0	BORING STARTED: 5/19/2017	BORING COMPLETED: 5/19/2017
DRILLING METHOD: 4" HSA	RIG TYPE: D-50	HAMMER: Automatic
GROUNDWATER (ft): Dry upon completion of soil augering	BORING DIAMETER (IN): 4	SHEET 1 OF 1

Remarks: Elevations interpolated from provided topographic mapping to nearest whole foot

Groundwater	ELEV. (FT.)	DEPTH (FT.)	MATERIAL DESCRIPTION	Lithology	Sample Type	Recovery (in)	RQD (%)	Qu	STANDARD PENETRATION RESISTANCE (N) 0 10 20 30 40 50	BLOWS /6"
	1003	0	Topsoil 8 inches							
	1002.3		Fat clay (CH), with black oxide nodules, STIFF to FIRM, light brown with gray mottling, moist			16			●	4 - 6 - 7
						12			●	3 - 3 - 4
						8			●	3 - 50/2
	998.0	5	Auger refusal encountered at 5.0 feet							
		10								
		15								
		20								

CRAIG2 1183-17-020 BLDG 198 BLUEGRASS STATION.GPJ QOR_CORP.GDT 6/13/17

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TEST BORING RECORD

BORING NO: **B-3**

PROJECT: Building 198 Bluegrass Station	JOB NO: 1183-17-020	REPORT NO:
PROJECT LOCATION: Lexington, KY		
ELEVATION: 1,009.0	BORING STARTED: 5/19/2017	BORING COMPLETED: 5/19/2017
DRILLING METHOD: 4" HSA	RIG TYPE: D-50	HAMMER: Automatic
GROUNDWATER (ft): Dry upon completion of soil augering	BORING DIAMETER (IN): 4	SHEET 1 OF 1

Remarks: Elevations interpolated from provided topographic mapping to nearest whole foot

Groundwater	ELEV. (FT.)	DEPTH (FT.)	MATERIAL DESCRIPTION	Lithology	Sample Type	Recovery (in)	RQD (%)	Qu	STANDARD PENETRATION RESISTANCE (N) 0 10 20 30 40 50	BLOWS /6"
	1009	0	Topsoil 8 inches							
	1008.3		Lean clay (CL), with black oxide nodules, STIFF, brown, moist			18			●	4 - 5 - 5
						14			●	4 - 5 - 9
	1005.5		Fat clay (CH), with black oxide nodules, STIFF to VERY STIFF, light brown, moist			16			●	6 - 7 - 7
		5				17			●	3 - 6 - 9
	1001.3		Auger refusal encountered at 7.7 feet							
		10								
		15								
		20								

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TEST BORING RECORD

BORING NO: **B-4**

PROJECT: Building 198 Bluegrass Station	JOB NO: 1183-17-020	REPORT NO:
PROJECT LOCATION: Lexington, KY		
ELEVATION: 994.0	BORING STARTED: 5/19/2017	BORING COMPLETED: 5/19/2017
DRILLING METHOD: 4" HSA	RIG TYPE: D-50	HAMMER: Automatic
GROUNDWATER (ft): Dry upon completion of soil augering	BORING DIAMETER (IN): 4	SHEET 1 OF 1

Remarks: Elevations interpolated from provided topographic mapping to nearest whole foot

Groundwater	ELEV. (FT.)	DEPTH (FT.)	MATERIAL DESCRIPTION	Lithology	Sample Type	Recovery (in)	RQD (%)	Qu	STANDARD PENETRATION RESISTANCE (N) 0 10 20 30 40 50	BLOWS /6"
	994	0	Topsoil 6 inches							
	993.5		Fat clay (CH), with black oxide nodules, FIRM to STIFF, light brown with gray mottling, moist			15			●	2 - 3 - 4
						10			●	4 - 4 - 5
		5				18			●	4 - 7 - 7
	988.5		Weathered limestone			5			●	50/5
						0			●	50/3
	983.5	10	Auger refusal encountered at 10.5 feet							
		15								
		20								

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TEST BORING RECORD

BORING NO: **B-5**

PROJECT: Building 198 Bluegrass Station	JOB NO: 1183-17-020	REPORT NO:
PROJECT LOCATION: Lexington, KY		
ELEVATION: 1,005.0	BORING STARTED: 5/19/2017	BORING COMPLETED: 5/19/2017
DRILLING METHOD: 4" HSA	RIG TYPE: D-50	HAMMER: Automatic
GROUNDWATER (ft): Dry upon completion of soil augering	BORING DIAMETER (IN): 4	SHEET 1 OF 1

Remarks: Elevations interpolated from provided topographic mapping to nearest whole foot

Groundwater	ELEV. (FT.)	DEPTH (FT.)	MATERIAL DESCRIPTION	Lithology	Sample Type	Recovery (in)	RQD (%)	Qu	STANDARD PENETRATION RESISTANCE (N) 0 10 20 30 40 50	BLOWS /6"
	1005	0	Topsoil 9 inches							
	1004.3		Fat clay (CH), with black oxide nodules, STIFF, brown, moist			18			●	4 - 5 - 7
	1003.0		Fat clay (CH), with black oxide nodules, STIFF, light brown with gray mottling			24		4,567		
		5				12			●	3 - 6 - 7
	998.6		Weathered limestone			5			●	50/5
	998.0		Auger refusal encountered at 7.0 feet							
		10								
		15								
		20								

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TEST BORING RECORD

BORING NO: **B-6**

PROJECT: Building 198 Bluegrass Station	JOB NO: 1183-17-020	REPORT NO:
PROJECT LOCATION: Lexington, KY		
ELEVATION: 1,006.0	BORING STARTED: 5/19/2017	BORING COMPLETED: 5/19/2017
DRILLING METHOD: 4" HSA	RIG TYPE: D-50	HAMMER: Automatic
GROUNDWATER (ft): Dry upon completion of soil augering	BORING DIAMETER (IN): 4	SHEET 1 OF 1

Remarks: Elevations interpolated from provided topographic mapping to nearest whole foot

Groundwater	ELEV. (FT.)	DEPTH (FT.)	MATERIAL DESCRIPTION	Lithology	Sample Type	Recovery (in)	RQD (%)	Qu	STANDARD PENETRATION RESISTANCE (N) 0 10 20 30 40 50	BLOWS /6"
	1006	0	Topsoil 6 inches							
	1005.5		FILL - Lean clay (CL), with black oxide nodules, STIFF, brown moist			18				3 - 4 - 5
	1004.2		FILL - Fat clay (CH), with black oxide nodules, STIFF, light brown, moist			14				6 - 7 - 8
	1003.0		FILL - Lean clay (CL), with possible buried topsoil, brick fragments, rock fragments, VERY STIFF, dark brown to brown, moist			18				6 - 6 - 9
	1001.8	5	Fat clay (CH), with black oxide nodules, VERY STIFF, light brown, moist							
	999.4		Auger refusal encountered at 6.6 feet			1				50/1
		10								
		15								
		20								

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FIELD TESTING PROCEDURES

Field Operations: The general field procedures employed by S&ME, Inc. are summarized in ASTM D 420 which is entitled "Investigating and Sampling Soils and Rocks for Engineering Purposes." This recommended practice lists recognized methods for determining soil and rock distribution and ground water conditions. These methods include geophysical and in situ methods as well as borings.

Borings are drilled to obtain subsurface samples using one of several alternate techniques depending upon the subsurface conditions. These techniques are:

a. Continuous 2-1/2 or 3-1/4 inch I.D. hollow stem augers;
b. Wash borings using roller cone or drag bits (mud or water);
c. Continuous flight augers (ASTM D 1425).

These drilling methods are not capable of penetrating through material designated as "refusal materials." Refusal, thus indicated, may result from hard cemented soil, soft weathered rock, coarse gravel or boulders, thin rock seams, or the upper surface of sound continuous rock. Core drilling procedures are required to determine the character and continuity of refusal materials.

The subsurface conditions encountered during drilling are reported on a field test boring record by a field engineer who is on site to direct the drilling operations and log the recovered samples. The record contains information concerning the boring method, samples attempted and recovered, indications of the presence of various materials such as coarse gravel, cobbles, etc., and observations between samples. Therefore, these boring records contain both factual and interpretive information. The field boring records are on file in our office.

The soil and rock samples plus the field boring records are reviewed by a geotechnical engineer. The engineer classifies the soils in general accordance with the procedures outlined in ASTM D 2488 and prepares the final boring records that are the basis for all evaluations and recommendations.

The final boring records represent our interpretation of the contents of the field records based on the results of the engineering examinations and tests of the field samples. These records depict subsurface conditions at the specific locations and at the particular time when drilled. Soil conditions at other locations may differ from conditions occurring at these boring locations. Also, the passage of time may result in a change in the subsurface soil and ground water conditions at these boring locations. The lines designating the interface between soil or refusal materials on the records and on profiles represent approximate boundaries. The transition between materials may be gradual. The final boring records are included with this report. The detailed data collection methods using during this study are discussed on the following pages.

Soil Test Borings: Soil test borings were made at the site at locations shown on the attached Boring Plan. Soil sampling and penetration testing were performed in accordance with ASTM D 1586.

The borings were made by mechanically twisting a 5-5/8" outer diameter auger into the soil. At regular intervals, the drilling tools were removed and samples obtained with a standard 1.4 inch I.D., 2 inch O.D., split tube sampler. The sampler was first seated 6 inches to penetrate any loose cuttings, then driven an additional foot with blows of a 140-pound hammer falling 30 inches. The number of hammer blows required to drive the sampler the final foot was recorded and is designated the "penetration resistance".

Representative portions of the samples, thus obtained, were placed in glass jars and transported to the laboratory. In the laboratory, the samples were examined to verify the driller's field classifications. Test Boring Records are attached which graphically show the soil descriptions and penetration resistances.

Soil Auger Soundings: Soil auger soundings were made at the site at the locations shown on the attached Boring Location Plan. The soundings were performed by mechanically twisting a steel auger into the soil. However, unlike the soil test borings, a smaller diameter solid stem auger was used and no split-spoon samples were obtained. The driller provided a general description of the soil encountered by observing the soils brought to the surface by the twisting auger. The auger was advanced until refusal materials were encountered and the refusal depth was noted by the driller. The auger is then withdrawn and the depths to water or caved materials are then measured and recorded by the driller.

Soil auger soundings provide a rapid, economical method of obtaining the approximate bedrock depth, groundwater depth, and general soil conditions at locations where detailed soil testing and sampling is not required.

Water Level Readings: Water table readings are normally taken in conjunction with borings and are recorded on the "Test Boring Records". These readings indicate the approximate location of the hydrostatic water table at the time of our field investigation. Where impervious soils are encountered (clayey soils) the amount of water seepage into the boring is small, and it is generally not possible to establish the location of the hydrostatic water table through water level readings. The ground water table may also be dependent upon the amount of precipitation at the site during a particular period of time. Fluctuations in the water table should be expected with variations in precipitation, surface run-off, evaporation and other factors.

The time of boring water level reported on the boring records is determined by field crews as the drilling tools are advanced. The time of boring water level is detected by changes in the drilling rate, soil samples obtained, etc. Additional water table readings are generally obtained at least 24 hours after the borings are completed. The time lag of at least 24 hours is used to permit stabilization of the ground water table which has been disrupted by the drilling operations. The readings are taken by dropping a weighted line down the boring or using an electrical probe to detect the water level surface. Occasionally the borings will cave-in, preventing water level readings from being obtained or trapping drilling water above the caved-in zone. The cave-in depth is also measured and recorded on the boring records.

Appendix III – Laboratory Data Summary

Form No. TR-2370-LEX-SUM-1
Revision No. : 2
Revision Date: 01/04/17

Lab Summary

S&ME

S&ME, Inc - Lexington 2020 Liberty Road, Suite 105, Lexington, KY 40505

Project No.: 1183-17-020 Report Date: 05/26/17

Project Name: Building 198 Bluegrass Station

Client Name: Finance and Administration (Kentucky)

Client Address: 403 Wapping Street, Frankfort, KY 40601

BORING NO.	SAMPLE DEPTH, FT.	SAMPLE TYPE	USCS	NATURAL MOISTURE CONTENT,%	ATT. LIMITS L.L.	ATT. LIMITS P.L.	ATT. LIMITS P. I.	APPROX % RET.ON #40	MAX DRY DENSITY, PCF @ OPT MC % (STD. PROCTOR)	WET UNIT WEIGHT, PCF	DRY UNIT WEIGHT, PCF	APPROX ROCK UNCONFINED COMPRESSIVE STRENGTH, PSI	SOIL UNCONFINED COMPRESSIVE STRENGTH, PSF	% FINER THAN NO. 200	INTER-POLATED AT 95% CBR, %
B-1	0.0 - 1.5	SPT		21.5											
B-1	2.0 - 4.0	UD	CH	31.7	76	29	47	<5		121.3	92.1		4,586		
B-1	4.0 - 5.5	SPT		29.2											
B-1	6.0 - 7.5	SPT		25.5											
B-1	8.5 - 8.8	SPT		16.8											
B-2	0.0 - 1.5	SPT		27.1											
B-2	1.5 - 3.0	SPT		31.3											
B-2	3.5 - 4.2	SPT		27.8											
B-3	0.0 - 1.5	SPT		31.7											
B-3	1.5 - 3.0	SPT	CL	24.3	49	22	27	5							
B-3	3.5 - 5.0	SPT		29.5											
B-3	6.0 - 7.5	SPT		23.1											

Notes: * - Gravel excluded. † - Gravel significant portion of sample and was included in MC. **Gravelly material, low recovery, insufficient for mc test.

Chisa Puckett
Technical Responsibility

x Chisa Puckett
Chisa Puckett

Senior Engineering Technician
Position

05/26/17
Date

Form No. TR-2370-LEX-SUM-1
Revision No. : 2
Revision Date: 01/04/17

Lab Summary

S&ME

S&ME, Inc - Lexington 2020 Liberty Road Suite 105, Lexington, KY 40505

Project No.: 1183-17-020 Report Date: 05/26/17

Project Name: Building 198 Bluegrass Station

Client Name: Finance and Administration (Kentucky)

Client Address: 403 Wapping Street, Frankfort, KY 40601

BORING NO.	SAMPLE DEPTH, FT.	SAMPLE TYPE	USCS	NATURAL MOISTURE CONTENT,%	ATT. LIMITS L.L.	ATT. LIMITS P.L.	ATT. LIMITS P. I.	APPROX % RET.ON #40	MAX DRY DENSITY, PCF @ OPT MC % (STD. PROCTOR)	WET UNIT WEIGHT, PCF	DRY UNIT WEIGHT, PCF	APPROX ROCK UNCONFINED COMPRESSIVE STRENGTH, PSI	SOIL UNCONFINED COMPRESSIVE STRENGTH, PSF	% FINER THAN NO 200	INTER-POLATED AT 95% CBR, %
B-4	0.0 - 1.5	SPT		26.2											
B-4	1.5 - 3.0	SPT		30.6											
B-4	3.5 - 5.0	SPT		25.3 *											
B-4	6.0 - 6.4	SPT		17.7 *											
B-4	8.5 - 8.9	SPT		8.5 *											
B-5	0.0 - 1.5	SPT		20.1											
B-5	2.0 - 4.0	UD	CH	29.3	72	26	46	<5		123.5	95.5		4,567		
B-5	4.0 - 5.5	SPT		27.5											
B-5	6.0 - 6.4	SPT		30.0 *											
B-6	0.0 - 1.5	SPT		21.3											
B-6	1.5 - 3.0	SPT		23.5											
B-6	3.5 - 5.0	SPT		29.8 *											

Form No. TR-D2166-01
Revision No. : 0
Revision Date: 3/29/13
Client Report No: 1

UNCONFINED COMPRESSIVE STRENGTH OF COHESIVE SOILS

S&ME

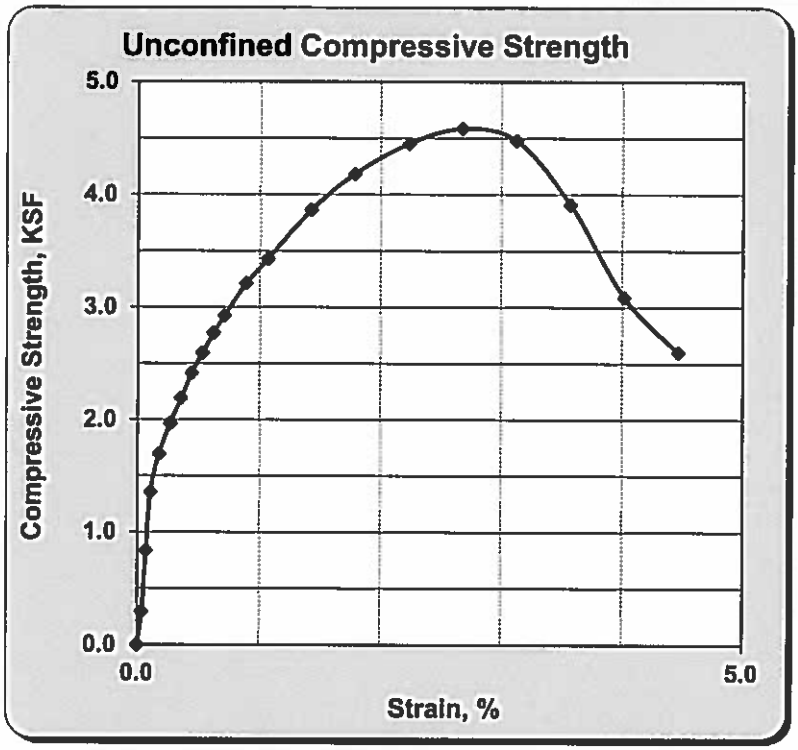
ASTM D2166

Quality Assurance

S&ME, Inc - Lexington 2020 Liberty Road, Suite 105 Lexington, KY 40505

Project No.:	1183-17-020	Report Date:	05/26/17
Project Name:	Building 198 Bluegrass Station	Test Date(s):	05/23/17
Client Name:	Finance and Administration (Kentucky)		
Client Address:	403 Wapping Street, Frankfort, KY 40601		
Location:	B-1 Depth (ft.): 3.4 - 3.9	Sample Date:	05/19/17

Sample Description:	Yellowish brown Fat clay				CH
Type and Specification	*S&ME ID #*	*Cal Date:*	*Type and Specification*	*S&ME ID #*	*Cal Date:*
Balance (0.01 gram)	23954	10/06/16			
Calipers (0.001 inches)	24319	05/21/16			
Load Frame	24004	02/01/17			



Failed Specimen

Type of Sample: UD
Source of Moisture Sample: 3/4 sample

Initial Dry Unit Weight: 92.1 pcf
Initial Water Content: 31.7%
Unconfined Compressive Strength, q_u: 4.586 KSF
Undrained Shear Strength, s_u: 2.293 KSF

Liquid Limit: 76
Plasticity Index: 47
Height to Diameter Ratio: 2.0
Rate of Strain (%/m): 1.1
Strain at Failure: 2.7

References / Comments / Deviations:

May 26 2017 11:15 AM

Chisa Puckett — *Technical Responsibility*

x Chisa Puckett — Chisa Puckett

Senior Engineering Technician — *Position*

5/26/2017 — *Date*

Form No. TR-D2166-01
Revision No. : 0
Revision Date: 3/29/13
Client Report No: 1

UNCONFINED COMPRESSIVE STRENGTH OF COHESIVE SOILS

ASTM D2166

S&ME

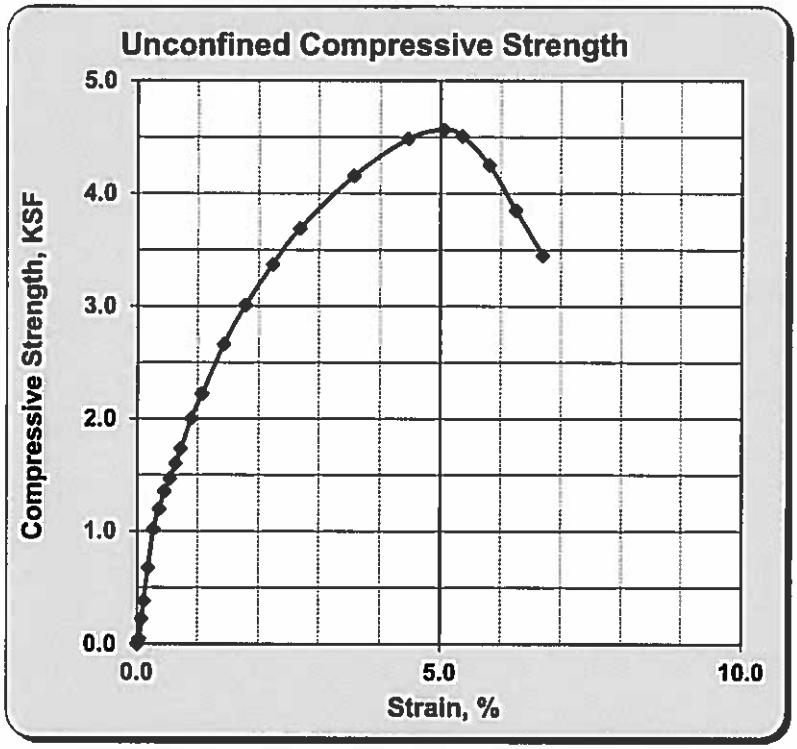
Quality Assurance

S&ME, Inc - Lexington 2020 Liberty Road, Suite 105 Lexington, KY 40505

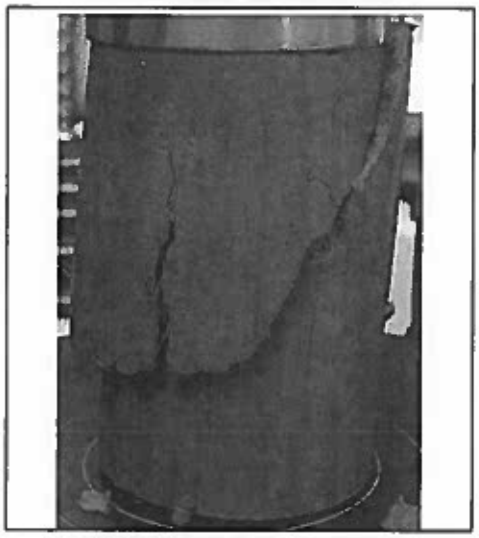
Project No.:	1183-17-020	Report Date:	05/26/17
Project Name:	Building 198 Bluegrass Station	Test Date(s):	05/23/17
Client Name:	Finance and Administration (Kentucky)		
Client Address:	403 Wapping Street, Frankfort, KY 40601		
Location:	B-5 Depth (ft.): 3.4 - 3.9	Sample Date:	05/19/17

Sample Description: Dark yellowish brown Fat clay — CH

Type and Specification	*S&ME ID #*	*Cal Date:*	*Type and Specification*	*S&ME ID #*	*Cal Date:*
Balance (0.01 gram)	23954	10/06/16			
Calipers (0.001 inches)	24319	05/21/16			
Load Frame	24004	02/01/17			



Failed Specimen



Type of Sample: UD
Source of Moisture Sample: 3/4 sample

Initial Dry Unit Weight: 95.5 pcf
Initial Water Content: 29.3%
Unconfined Compressive Strength, q_u: 4.567 KSF
Undrained Shear Strength, s_u: 2.283 KSF

Liquid Limit: 72
Plasticity Index: 46
Height to Diameter Ratio: 2.0
Rate of Strain (%/m): 1.1
Strain at Failure: 5.1

References / Comments / Deviations:

May 26 2017 11:16 AM

Chisa Puckett — *Technical Responsibility*

Chisa Puckett

Senior Engineering Technician — *Position*

5/26/2017 — *Date*

LABORATORY TESTING PROCEDURES

Soil Classification: Soil classifications provide a general guide to the engineering properties of various soil types and enable the engineer to apply past experience to current problems. In our investigations, samples obtained during drilling operations are examined in our laboratory and visually classified by an engineer. The soils are classified according to consistency (based on number of blows from standard penetration tests), color and texture. These classification descriptions are included on our "Test Boring Records."

The classification system discussed above is primarily qualitative and for detailed soil classification two laboratory tests are necessary: grain size tests and plasticity tests. Using these test results the soil can be classified according to the AASHTO or Unified Classification Systems (ASTM D 2487). Each of these classification systems and the in-place physical soil properties provides an index for estimating the soil's behavior. The soil classification and physical properties obtained are presented in this report.

Compaction Tests: Compaction tests are run on representative soil samples to determine the dry density obtained by a uniform compactive effort at varying moisture contents. The results of the test are used to determine the moisture content and unit weight desired in the field for similar soils. Proper field compaction is necessary to decrease future settlements, increase the shear strength of the soil and decrease the permeability of the soil.

The two most commonly used compaction tests are the Standard Proctor test and the Modified Proctor test. They are performed in accordance with ASTM D 698 and D 1557, respectively. Generally, the Standard Proctor compaction test is run on samples from building or parking areas where small compaction equipment is anticipated. The Modified compaction test is generally performed for heavy structures, highways, and other areas where large compaction equipment is expected. In both tests a representative soil sample is placed in a mold and compacted with a compaction hammer. Both tests have four alternate methods.

Test	Method	Hammer Wt./Fall	Mold Diam.	Run on Matl. Finer Than	No. of Layers	No. of Blows/Layer
Standard	A	5.5 lb./12"	4"	No. 4 sieve	3	25
D 698	B	5.5 lb./12"	4"	3/8" sieve	3	25
	C	5.5 lb./12"	6"	3/4" sieve	3	56

Test	Method	Hammer Wt./Fall	Mold Diam.	Run on Matl. Finer Than	No. of Layers	No. of Blows/Layer
Modified	A	10 lb./18"	4"	No. 4 sieve	5	25
D 1557	B	10 lb./18"	4"	3/8" sieve	5	25
	C	10 lb./18"	6"	3/4" sieve	5	56

The moisture content and unit weight of each compacted sample is determined. Usually 4 to 5 such tests are run at different moisture contents. Test results are presented in the form of a dry unit weight versus moisture content curve. The compaction method used and any deviations from the recommended procedures are noted in this report.

Atterberg Limits: Portions of the samples are taken for Atterberg Limits testing to determine the plasticity characteristics of the soil. The plasticity index (PI) is the range of moisture content over which the soil deforms as a plastic material. It is bracketed by the liquid limit (LL) and the plastic limit (PL). The liquid limit is the moisture content at which the soil becomes sufficiently "wet" to flow as a heavy viscous fluid. The plastic limit is the lowest moisture content at which the soil is sufficiently plastic to be manually rolled into tiny threads. The liquid limit and plastic limit are determined in accordance with ASTM D 4318.

Moisture Content: The Moisture Content is determined according to ASTM D 2216.

Exhibit F

(Environmental Site Assessment to be distributed to Certified List through U.S. Postal Services)

Exhibit G

RETURN THIS FORM FILLED OUT WITH PHASE I SUBMITTAL

MINORITY BUSINESS ENTERPRISE (MBE) PARTICIPATION

1.01 **CERTIFICATION OF MBE:** Any MBE utilized pursuant to this Section shall be certified as an MBE by one of the following: Kentucky Transportation Cabinet or other state Transportation agencies, the Louisville Metropolitan Sewer District, the Kentucky Minority Supplier Development Council or other state Minority Supplier Development Councils, the Kentucky Certification Cooperative, or the Small Business Administration.

1.02 **OBLIGATION OF BIDDER/CONTRACTOR:** Bidder/Contractor shall make a good faith effort to meet the MBE contract goal set by the Commonwealth by including MBE's as subcontractors and/or material suppliers on 10% of the total estimated cost of the Contract. The failure to meet the foregoing goal shall not result in disqualification from bidding or being awarded a contract. However, Bidders/Contractors not meeting the MBE goal shall be expected to provide written proof of their good faith efforts. Award of the contract shall be conditioned upon satisfaction of the requirements established by this section. The Bidder/Contractor shall attempt to divide the work in the contract to facilitate use of MBE's (however, there is no requirement that the work be artificially divided or divided in a way that raises the bid price of the Bidder/Contractor).

1.03 **PROOF REQUIRED:** Each bidder shall furnish written proof in their bid package that they reached the MBE participation goal for this Contract, or of their good faith efforts to meet the MBE participation goal. A copy of each participating MBE's certification shall accompany the required forms. All submissions shall be subject to verification of the Commonwealth.

A. Proof that the apparent successful bidder reached the MBE goal shall consist of the following and shall be made on form MB-2-A, attached hereto:

1. The names and addresses of MBE firms that will participate in the contract;
2. A description of the work each named MBE firm will perform;
3. The dollar amount of participation by each named MBE firm;
4. The percentage amount of participation by each named MBE firm;

B. Proof that the apparent successful bidder made a good faith efforts to meet the MBE participation goal may include the following:

1. Advertisement by the Bidder/Contractor of MBE contracting opportunities associated with this contract in at least one of each of the following periodicals: a periodical in general circulation throughout the Commonwealth, a trade periodical focused on MBE contractors/suppliers in general circulation throughout the Commonwealth, and a minority-focused periodical in general circulation throughout the Commonwealth. The Bidder/Contractor shall include copies of the dated advertisements in his bid package;

2. Written notice of MBE opportunities in this contract to at least five pertinent MBE's at least seven days prior to the bid opening date. Copies of the written notices shall be included in the bid package;

3. The Bidder/Contractor's response(s) to those MBE's who requested plans, specifications and/or contracting requirements. Copies of said responses shall be included in the bid package;

4. Documentation on form MB-2-B of good faith negotiations with at least three MBE's, with no rejection of a qualified MBE without sound reason, including price quotes that are above other subcontractor's price quotes;

5. Utilization of the Finance and Administration Cabinet's Office of Equal Employment Opportunity and Contract Compliance for referrals to organizations that assist in locating MBE"s. Proof of use of such referrals and contacts made as a result thereof shall be included in the bid package.

MB-2-A

MINORITY BUSINESS AVAILABILITY VERIFICATION

_____ does commit itself that on the following project:
NAME OF COMPANY

_____ _____ _____
PROJECT NAME BID INVITATION NUMBER ENGINEERING FILE NUMBER

The Bidder agrees to furnish information required by the Commonwealth of Kentucky to indicate the Minority Business which it intends to utilize. Breach of this commitment constitutes breach of the Bidder's contract if awarded.

NAME OF MINORITY BUSINESS	TELEPHONE	TYPE OF WORK

DOLLAR VALUE	PERCENT	MINORITY CLASSIFICATION

The undersigned shall enter into a formal agreement with the minority business firms for work listed in this schedule conditioned upon execution of a contract with the Commonwealth of Kentucky.

Minority business firms listed above by the Bidder and accepted by the Owner and the Architect/Engineer shall be used on the work for which they were proposed and accepted and shall not be changed except with the written approval of the Owner and the Architect/Engineer. The undersigned hereby certifies that he or she has read the terms of this commitment and is authorized to bind the Bidder to the commitment herein set forth.

Signature and title of authorized official of the company and the data shall be properly executed on this document or the bid will be deemed nonresponsive.

_____ _____
NAME OF AUTHORIZED OFFICER **TITLE**

_____ _____
SIGNATURE **DATE**

If you are bidding as a General Contractor on this project i.e. direct bidding and a Minority as defined herein, please provide a copy of your MBE Certification.

Submit with Bid.
(Please copy additional Minority Business Availability Forms as necessary.)
MB-2-B

MINORITY BUSINESS UNAVAILABILITY VERIFICATION

I, _____, _____
(TITLE)

of _____
(PRIME BIDDER)

certify that on _____ I contacted the following minority owned business by: (circle one) Certified Mail, Phone, In Person to obtain a bid for work items to be performed on the Contract.

MINORITY CONTRACTOR	WORK ITEMS SOUGHT	FORM OF BID SUPPORT (I.E., UNIT PRICE, MATERIALS LABOR & LABOR ONLY)

To the best of my knowledge and belief, said minority owned business was unavailable (exclusive of unavailability due to lack of agreement on price) for work on this project, or unable to prepare a bid, for the following reason(s):

_____

_____

_____

_____

_____

_____

SIGNATURE _____

DATE _____

_____ was offered an
(NAME OF MINORITY BUSINESS)

opportunity to bid on the above-identified work on _____ by

_____
(SOURCE)

The above statement is a true and accurate account of why I did not submit a bid on this project.

_____
(SIGNATURE OF MINORITY BUSINESS)

_____
(TITLE) (DATE)

Submit with Bid if Applicable.
(Please copy additional Minority Business Unavailability Forms as needed.)

Exhibit H

OWNERSHIP DISCLOSURE STATEMENT

I. Please list below all persons that have an ownership interest in this property leased by the Commonwealth. If the property owner is a corporation, business trust, or partnership (per KRS 56:809), list the name of the corporation, business trust, or partnership and then list all persons having five percent (5%) or more ownership interest in such entities to include silent or limited partners. The lessor furthermore agrees to notify the Commonwealth of all persons involved in any change or transfer of ownership of five percent (5%) or more to include silent or limited partners. Non-compliance may result in termination of the lease agreement.

Identify Lessor as:

☐ Individual ☐ Sole Proprietorship ☐ Partnership/Joint Venture ☐ Estate

☐ Corporation ☐ Public Service Corp. ☐ Government/Non Profit Agency

Identify Social Security Number or Federal ID Number for Lessor:_____

Name	Home Address	Social Security Number	% of Ownership

(if more space is needed, please attach separate sheet)

II. Are there any of the owners of t his lease, listed heretofore, and/or their immediate relatives (Father, Mother, Sister, Brother, Son, Daughter, Spouse) an officer or employee of any state agency, board, commission, etc..?

☐ YES ☐ NO If yes, please list:

Owner	Full Name of Relative & Social Security Number	Agency, Board or Commission	Title

(if more space is needed, please attach separate sheet)

III. We are submitting the information requested and certify it to be accurate:

AUTHORIZED AGENT OF THE LESSOR: _____

TITLE:_____ **TYPED NAME:**_____

BUSINESS ADDRESS:_____

_____

DAYTIME TELEPHONE:_____ _____

**** NOTARY:**_____ **NOTARIZED THIS DATE:**_____

COMMISSION EXPIRES:_____ **COUNTY OF:**_____

Did you fill out all the blanks? Please recheck.** Form notarized with SEAL?

Exhibit I

Commonwealth of Kentucky
SOLICITATION

TITLE: RFP 102317 Bluegrass Station Bldg 352 Notice

DATE ISSUED	SOLICITATION CLOSES	SOLICITATION NO.
08/03/2017 **RECORD DATE** 08/03/2017	**Date:** 10/23/17 **Time:** 4:00 p.m.	RFP 785 1800000032 Phases I & II

ISSUED BY	ADDRESS TO
Finance Div of Real Property Nancy Brownlee	RP Division of Real Properties Bush Building, Third Floor, 403 Wapping Street FRANKFORT KY 40601 US

V E N D O R

ORDER ADDRESS	PAYMENT ADDRESS
Name:	**Name:**
Address:	**Address:**
City, State Zip Code:	**City, State Zip Code:**
Phone #:	**Phone #:**
Fax #:	**Fax #:**
Email Address:	**Email Address:**
Contact Name:	**Contact Name:**
Contact Email:	**Contact Email:**
Vendor Customer (VC) #:	**Vendor Customer (VC) #:**

FOR INFORMATION CALL:

Nancy Brownlee
502-564-2205

ONLINE BIDDING PROHIBITED

yes

OWNERSHIP TYPE:

☐ **Sole Proprietorship** ☐ **Partnership** ☐ **Corporation**

SIGNATURE OF AUTHORIZED AGENT IS <u>REQUIRED</u> UNLESS RESPONSE IS SUBMITTED ELECTRONICALLY.

FAILURE TO SIGN SHALL RENDER THE BID INVALID.

Signature X_____ **FEIN#**_____ **DATE**_____

All offers subject to all terms and conditions contained in this solicitation.

Line Items

1 Commodity Group Default

Line	CL Description	Due Date	Quantity	Unit Issue	Unit Cost	Line Total Or Contract Amt
1	RFP 102317 Bluegrass Station Bldg 352 Notice		0.00			

Comm Code	Comm Description	Manufacturer	Model #	Manuf Part #
97100	REAL PROPERTY RENTAL OR LEASE			

Extended Description

RFP102317-Design, Build, Finance Built to Suit RFP Lease Contract for Approx 6.02 acres of land to private entity to Finance, Design, Develop, Construct and Lease Back to the Commonwealth Bldg 352 a 64,000 sqft maintenance and training facility (rotary wing aircraft hangar) and related infrastructure

BILL To	SHIP To
421083 FAC FSS REAL PROPERTIES 403 WAPPING STREET BUSH BLDG 3RD FLOOR FRANKFORT KY 40601-2607 US	

Evaluation Criteria			
The following criteria will be used when determining the award of this solicitation			
Code	**Criteria Description**	**Points**	**Vendor Response** *(DO NOT LIST PRICES IN THIS SECTION. UNIT PRICES AND TOTAL PRICES MUST BE FILLED IN ADJACENT TO THEIR LINE ITEMS.)*

To access the RFP document, click on the attachments button and download the attachments.

Exhibit J

PARTIAL PERMITS

The Building Code Official is authorized to issue a permit for any part of a building or structure before the plan complete package has been submitted. Upon request, the Building Code Official is also authorized to issue partial permits if an entire plan package has been filed but falls short of meeting the requirements for full permit.

The issuance of a partial permit is contingent upon adequate information and details having been filed to demonstrate compliance with all pertinent requirements of the Code.

SITE / FOUNDATION

The following items are required before a Site and Foundation Permit is to be issued. Not all items will be applicable on each project. All drawings shall be dimensioned and drawn to scale.

- ☐ **ARCHITECT/ENGINEER**
  The services of an Architect or Engineer shall be confirmed. When their services are required, the plans shall bear the seal and signature of the Architect and /or Engineer (KRS 322/323 and Table 122.1 of the KBC)

- ☐ **SITE PLAN**
  A site plan showing the location of the building and its distance to property lines and other buildings on the same property and finished grades shall be submitted. (Section 106.2 KBC)

- ☐ **SITE SURVEY**
  A plan illustrating the location of property lines and bearing the seal and signature of a land surveyor shall be submitted. (Section 106.2 KBC)

- ☐ **NOTICE:**
  Information for the installation of underground sprinkler supply lines shown on the site plan is not covered under a site and foundation permit. A separate letter of approval or disapproval shall be required for this work. This work shall be performed by a Kentucky licensed sprinkler contractor. (refer to Krs 198B.560)

- ☐ **FOUNDATION PLAN**
  A foundation plan and details shall be submitted, including anchorage details. This includes final anchor bolt plans from pre-engineered metal buildings,

- ☐ **FLOOR PLANS**
  A floor plan of the building with sufficient information to identify all areas and the Use Group shall be submitted. (Chapter 3, KBC)

- ☐ **Seismic Design Data & Letter of Special Inspection (Sections 1603.1.5 and 1704)**

- ☐ **CONSTRUCTION TYPE**
  Sufficient construction .details (i.e. exterior walls, interior bearing structure and floor/roof assembly) shall be submitted to confirm the building will comply with the minimum construction required. (Chapters 5 and 6 KBC)

- ☐ **FIRE WALLS**
  If a fire all is provided, the location of this wall shall be identified on the foundation and floor plans. A full height section through the wall shall be submitted.

FIRE WALL- A wall designed with a noncombustible material, specified fire rating and structurally independent to allow collapse of construction on either side without causing collapse of the wall itself. A fire wall shall be continuous from footer to or through roof. (Section 705. KBC)

- ☐ **SUPPRESSION SYSTEM**
  Fire suppression design criteria shall be submitted when the project requires a sprinkler system involving more than 10 sprinklers. This applies to limited are systems as well as full coverage systems. (Section 903.2 and 302.1.1, KBC)

- ☐ **ADDITIONS**
  If the proposed structure is an addition to an existing structure, information confirming the following shall be submitted for the existing building: (Section 3403 KBC)

  a) Construction Type;
  b) Fire wall location, construction and fire rating;
  c) Building Area
  d) Number Stories
  e) Use group Classification: and
  f) Type suppression system (Full coverage of limited area).

- ☐ **FEE**
  The architectural plan review fee shall be paid in full before any release for construction can be issued. (Section 121 KBC)

- ☐ **FAST TRACK ELECTIVE**
  For applicants seeking a quicker footing and foundation review only. The drawings and documents identified above submitted by close of business any Wednesday, for a S/F review the following Friday afternoon. Fee shall be calculated from Table 121.3.1 ***plus*** an additional 50% of the full fee. Additional fee shall not be less than $400 and not more than $3000.